GENERAL CONDITIONS

1. THESE STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH ALL OTHER DRAWINGS, SPECIFICATIONS AND CONTRACT DOCUMENTS.
2. THE DESIGN, CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT SHALL CONFORM TO THE REFERENCE CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN. THE LATEST EDITIONS SHALL APPLY UNLESS NOTED OTHERWISE.
3. THE CONTRACTOR IS RESPONSIBLE FOR ALL CONSTRUCTION METHODS, PROCEDURES, AND SEQUENCES AND SHALL TAKE ALL NECESSARY MEANS TO MAINTAIN AND PROTECT THE STRUCTURAL INTEGRITY OF ALL CONSTRUCTION AT ALL STAGES. ALL FORMWORK, SHORING AND RESHORING, TEMPORARY BRACING AND SUPPORT, AND RECOMMENDED ERECTION PROCEDURES FOR ALL WORK UNTIL PERMANENT BRACING IS IN PLACE IS THE RESPONSIBILITY OF THE CONTRACTOR. IF NECESSARY SHORING, TEMPORARY BRACING, AND TEMPORARY SUPPORT SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER, REGISTERED IN THE PROJECT'S JURISDICTION.
4. COORDINATION WITH THE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS TO VERIFY THE LOCATIONS AND SIZES OF ALL CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, AND OTHER PROJECT REQUIREMENTS ARE THE CONTRACTOR'S RESPONSIBILITY. ALL REQUIRED OPENINGS, SLEEVES, OR OTHER COMPONENTS MAY NOT BE SHOWN ON THE STRUCTURAL DRAWINGS.
5. ALL INFORMATION OF EXISTING CONSTRUCTION, INCLUDING CONSTRUCTION COMPOSITION AND DIMENSIONS, WAS TAKEN FROM OWNER FURNISHED DRAWINGS OF PAST CONSTRUCTION AND SHALL BE FIELD VERIFIED. DEVIATIONS FROM THOSE SHOWN SHALL BE FURNISHED TO THE ARCHITECT AND ENGINEER.

PRE-ENGINEERED METAL BUILDINGS

1. DESIGN OF PRE-ENGINEERED METAL BUILDING SHALL BE IN ACCORDANCE WITH PROCEDURES SET FORTH IN MBMA'S "METAL BUILDING SYSTEMS MANUAL."
2. DESIGN LOADS SHALL BE A INDICATED ON THESE DRAWINGS AND IN ACCORDANCE WITH ASCE 7, "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" AND THE INTERNATIONAL BUILDING CODE, 2021, COORDINATE COLLATERAL LOAD DESIGN REQUIREMENTS WITH G.C., SUBCONTRACTORS AND EQUIPMENT.
3. ALL COLUMN BASES SHALL BE DESIGNED AS PINNED BASE. NO FIXED BASE DESIGNS ARE PERMITTED.
4. FOUNDATION SIZES INDICATED ON THE DRAWINGS ARE FOR PRICING ONLY AND ARE BASED ON PRELIMINARY REACTIONS. CONTRACTOR SHALL PROVIDE COLUMN REACTIONS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO BEGINNING FABRICATION OF THE BUILDING.
5. COORDINATE ALL PENETRATIONS THROUGH ROOF WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND. PLUMBING DRAWINGS AND ALL TRADES.
6. PROVIDE TEMPORARY SHORES, GUYS, BRACES AND OTHER SUPPORTS DURING ERECTION TO KEEP STRUCTURAL FRAMING SECURE, PLUMB, AND IN ALIGNMENT AGAINST TEMPORARY CONSTRUCTION LOADS AND LOADS EQUAL IN INTENSITY TO DESIGN LOADS. REMOVE TEMPORARY SUPPORTS WHEN PERMANENT STRUCTURAL FRAMING, CONNECTIONS, AND BRACING ARE IN PLACE, UNLESS OTHERWISE INDICATED. COMPLY WITH ALL LOCAL, STATE, FEDERAL AND OSHA REGULATIONS PERTAINING TO ERECTION OF METAL BUILDING.

DESIGN CRITERIA

1. BUILDING CODE: THE PENNSYLVANIA UNIFORM CONSTRUCTION CODE, AND BY REFERENCE THE INTERNATIONAL BUILDING CODE 2021, AND ASCE 7-16 (BY REFERENCE OF IBC)
2. DESIGN GRAVITY LOADS:
   ROOF DEAD: 20 psf ROOF LIVE: 30 psf
3. ROOF SNOW LOAD:
   GROUND SNOW LOAD, P_g = 30 psf
   EXPOSURE FACTOR, C_e = 1.0
   IMPORTANCE FACTOR, I = 1.0
   THERMAL FACTOR, C_t = 1.0
   FLAT ROOF SNOW, $P_f = 0.7(P_g)(C_e)(I)(C_t)$ = 21 psf
4. WIND LOAD:
   BUILDING IMPORTANCE CATEGORY, II
   BASIC WIND SPEED (GUSTS), V = 112 mph
   EXPOSURE CATEGORY, C
   MEAN ROOF HEIGHT, H = 17.5 ft
   INTERNAL PRESSURE COEFFICIENT: $GC_{pi} = 0.18\pm$
   MAIN WIND FORCE RESISTING SYSTEM:
   MAXIMUM EFFECTIVE HORIZONTAL PRESSURE, P_w = 28.4 psf
   COMPONENTS AND CLADDING:
   MAX. EFFECTIVE WALL PRESSURE, P_{ecc} = 31.2 psf
   MAX. EFFECTIVE ROOF PRESSURE, P_{ecc} = 63.7 psf
   NET ROOF UPLIFT PRESSURE, P_u = 30.7 psf
5. EARTHQUAKE DESIGN DATA:
   SEISMIC USE GROUP, II
   SEISMIC IMPORTANCE FACTOR, I_e = 1.0
   MAPPED SPECTRAL RESPONSE ACCELERATIONS:
   SHORT PERIOD, S_S = 0.135
   ONE SECOND, S_1 = 0.043
   SPECTRAL RESPONSE COEFFICIENT,
   SHORT PERIOD, S_{DS} = 0.44
   ONE SECOND, S_{D1} = 0.68
   SITE CLASSIFICATION, D
   SITE COEFFICIENTS,
   SHORT PERIOD, F_a = 1.6
   ONE SECOND, F_v = 2.4
   SEISMIC DESIGN CATEGORY, B
   SEISMIC RESISTING SYSTEM: LOAD BEARING WALLS w/ SHEAR PANELS
   RESPONSE MODIFICATION FACTOR, R = 6.5
   DEFLECTION AMPLIFICATION FACTOR, C_d = 4
   SEISMIC RESPONSE COEFFICIENT, C_s = 0.18
   TOTAL DESIGN BASE SHEAR, V = 4 kips
   ANALYSIS PROCEDURE: EQUIVALENT LATERAL-FORCE METHOD

FOUNDATIONS / GEOTECHNICAL REPORT:

1. FOUNDATIONS HAVE BEEN DESIGNED WITHOUT ANY GEOTECHNICAL INFORMATION BASED ON ASSUMPTIONS FROM THE ENGINEER'S EXPERIENCE. NO NEW GEOTECHNICAL REPORT HAS BEEN PROVIDED BY THE OWNER FOR THIS PROJECT.
2. FOUNDATIONS AND/OR SLABS ON GRADE ARE TO BE PLACED ON INSITU SOILS AND HAVE BEEN DESIGNED FOR A NET ALLOWABLE BEARING PRESSURE OF 1500 psf.
3. BEARING ELEVATIONS ARE ESTIMATED. DETERMINATION OF ACTUAL BEARING ELEVATIONS AND FIELD CERTIFICATION OF ALLOWABLE BEARING PRESSURE SHALL BE PERFORMED BY AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO PLACING FOUNDATIONS.
4. FOUNDATIONS SHALL BEAR A MINIMUM OF 3 FEET BELOW FINISHED GRADE.
5. ALL UTILITY LINES SHALL BE SLEEVED THROUGH FOUNDATION WALLS, STEP FOOTINGS PER THE TYPICAL DETAIL AS NECESSARY FOR UTILITIES. COORDINATE ALL UTILITY LOCATIONS WITH THE MECHANICAL, ELECTRICAL, AND PLUMBING CONTRACTORS. UTILITY LINES SHALL NOT BE PLACED THROUGH OR UNDER FOOTINGS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL.

CAST IN PLACE CONCRETE:

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE MOST RECENT ADDITIONS AND REVISIONS REFERENCED BY THE CURRENT BUILDING CODE OF THE FOLLOWING CODES AND STANDARDS:
   "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI-318", AMERICAN CONCRETE INSTITUTE (ACI)
   "MANUAL OF STANDARD PRACTICE", CONCRETE REINFORCING STEEL INSTITUTE (CRSI)
2. MATERIAL SHALL CONFORM TO THE FOLLOWING STANDARDS:

CEMENT	ASTM C150, TYPE I OR II
AGGREGATES, NORMAL WEIGHT	ASTM C33
DEFORMED REINFORCING BARS	ASTM A615, GRADE 60, Fy = 60 ksi
WELDED WIRE FABRIC	ASTM A1064

3. CONCRETE STRENGTHS SHALL BE AS FOLLOWS:

LOCATION	EXPOSURE CLASS	F'c @ 28 DAYS	WEIGHT	MAX. W/C RATIO
PERIMETER FOUNDATIONS	F1	4000 psi	145 pcf	0.55
INTERIOR SLABS	F0	3000 psi	145 pcf	0.60
EXTERIOR STRUCTURAL CONCRETE	F2	4500 psi	145 pcf	0.45
MOISTURE & FREEZE EXPOSURE	F3	4500 psi	145 pcf	0.45
ALL OTHER STRUCTURAL INTERIOR CONCRETE	F0	3000 psi	145 pcf	0.60

4. ALL CONCRETE OF EXPOSURE CLASSES F1, F2 AND F3 SHALL MEET THE TOTAL AIR CONTENT ESTABLISHED IN TABLE 19.3.3.1 OF THE ACI BASED ON THE A MAXIMUM NOMINAL AGGREGATE SIZE.

MAX. NOMINAL AGGREGATE SIZE	TOTAL AIR EXPOSURE F1	TOTAL AIR EXPOSURE F2 OR F3
3/8"	6.0	7.5
1/2"	5.5	7.0
3/4"	5.0	6.0
1"	4.5	6.0

5. ALL CONCRETE OF EXPOSURE CLASS F3 SHALL MEET THE LIMITS OF TABLE 26.4.2.2(b) FOR THE QUANTITY OF POZZOLANS, INCLUDING FLY ASH AND SILICA FUME, AND SLAG.
6. CONCRETE PROPORTIONING SHALL BE ESTABLISHED BY THE SUPPLIER BASED UPON ACCEPTABLE TEST RECORDS MEETING THE ACI REQUIREMENTS.
7. SLABS ON GRADE SHALL BE PLACED ON 4" MINIMUM OF WELL GRADED, CRUSHED STONE OVER A 10 MIL. VAPOR RETARDER ON COMPACTED SUBGRADE.
8. ALL SLABS ON GRADE SHALL BE CONSTRUCTED TO THE FOLLOWING FLOOR FLATNESS/LEVELNESS (FF/FL) REQUIREMENTS: FF35/FL25.
9. WELDED WIRE FABRIC SHALL BE LAPPED TWO FULL MESH PANELS AND WIRED TOGETHER. FIBROUS REINFORCED CONCRETE SLABS SHALL BE REINFORCED WITH MACRO-SYNTHETIC FIBERS OF SUFFICIENT QUANTITY TO PROVIDE TEMPERATURE AND SHRINKAGE CRACK CONTROL PER THE MANUFACTURERS RECOMMENDATIONS.
10. CHAMFER ALL EXPOSED CONCRETE CORNERS 1/2" UNLESS SHOWN OTHERWISE ON THE ARCHITECTURAL DRAWINGS.

MASONRY:

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE MOST RECENT ADDITIONS AND REVISIONS REFERENCED BY THE CURRENT BUILDING CODE OF THE FOLLOWING CODES AND STANDARDS:
   "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", (TMS 402/602).
   "SPECIFICATIONS FOR MASONRY STRUCTURES", (TMS 402/602).
2. MATERIAL SHALL CONFORM TO THE FOLLOWING STANDARDS:

CONCRETE MASONRY UNITS	ASTM C90 NORMAL WEIGHT MIN. COMPRESSIVE STRENGTH ON NET AREA = 2000 psi (@ 28 DAYS)
CMU GROUT	ASTM C476, MIN. COMPRESSIVE STRENGTH = 2800 psi (@ 28 DAYS)
PRISM STRENGTH	f'm = 2000 psi PER TMS 402/602 - UNIT STRENGTH METHOD
HORIZONTAL JOINT REINF.	ASTM A951, 9 GAGE GALVANIZED TRUSS TYPE
REINFORCING BARS	ASTM A615, GRADE 60, Fy = 60,000 psi
MORTAR	ASTM C270, TYPE S

3. PROVIDE GALVANIZED HORIZONTAL JOINT REINFORCING IN ALL WALLS AT 16"o.c. UNLESS NOTED OTHERWISE. PROVIDE ONE-PIECE PREFABRICATED "T" AND "L" SHAPED UNITS AT 8"o.c. AT ALL WALL CORNERS AND INTERSECTIONS WITH HORIZONTAL JOINT REINFORCING.
4. IN GROUTED AND/OR REINFORCED MASONRY WALLS, USE MASONRY UNITS WITH CORES THAT ALIGN VERTICALLY. PROVIDE CONTINUOUS UNOBSTRUCTED CELLS FOR REINFORCEMENT PLACEMENT AND GROUTING.
5. ALL REINFORCING STEEL SHALL BE PLACED AND HELD IN THE PROPER POSITION WITH REINFORCING BAR POSITIONERS AS THE WALLS ARE CONSTRUCTED. LOWERING THE BARS IN FROM THE TOP OF A COMPLETED WALL OR SECTION OF WALL IS NOT ALLOWED.
6. ALL CELLS WITH VERTICAL REINFORCING BARS SHALL BE GROUTED SOLID. CELLS TO BE GROUTED SHALL BE KEPT CLEAN AND FREE OF OBSTRUCTIONS. GROUT POURS SHALL BE MADE IN LIFTS LESS THAN 5 FEET, AND SHALL NOT EXCEED 12 FEET. ALL GROUTED CELLS SHALL BE MECHANICALLY VIBRATED.
7. LAP SPLICES FOR REINFORCING BARS SHALL BE 50 BAR DIAMETERS UNLESS NOTED OTHERWISE.

WOOD FRAMING

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE MOST RECENT ADDITIONS AND REVISIONS REFERENCED BY THE CURRENT BUILDING CODE OF THE FOLLOWING CODES AND STANDARDS:
   "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", AMERICAN FOREST & PAPER ASSOC.
   "STRUCTURAL GLUE LAMINATED TIMBER", AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
   "U.S. PRODUCT STANDARD PS1 FOR CONSTRUCTION & INDUSTRIAL PLYWOOD", AMERICAN PLYWOOD ASSOCIATION
2. MATERIAL PROPERTIES SHALL BE AS FOLLOWS:
   LUMBER: SPRUCE-PINE-FIR No. 2 GRADE *or* BETTER (NELMA, NLGA, WCLIB, *or* WWPA)
   F_b = 875 psi F_v = 135 psi F_c = 1150 psi E = 1,400,000 psi
   ENGINEERED LUMBER:
   LAMINATED-VENEER LUMBER (LVL) ARE COMPOSITES OF WOOD VENEERS WITH THE GRAIN PRIMARILY PARALLEL TO MEMBER LENGTHS, BONDED TOGETHER WITH EXTERIOR-TYPE ADHESIVE COMPLYING WITH ASTM D2559. ALLOWABLE DESIGN VALUES DETERMINED ACCORDING TO ASTM D5456.
   SHEATHING: DOUGLAS FIR-LARCH OR DOUGLAS FIR-LARCH (NORTH)
   PLYWOOD: DOC PS 1, UNLESS OTHERWISE INDICATED WITH EXTERIOR, EXPOSURE 1 RATING
3. PROVIDE LUMBER FACTORY MARKED ON EACH PIECE WITH GRADE STAMP OF GRADING AGENCY. PROVIDE WOOD DECKING WITH EACH PIECE FACTORY MARKED WITH GRADE STAMP OF INSPECTION AGENCY EVIDENCING COMPLIANCE WITH GRADING RULE REQUIREMENTS AND IDENTIFYING GRADING AGENCY, SPECIES, GRADE, MOISTURE CONTENT AT TIME OF SURFACING, AND MILL.
4. LUMBER SHALL BE PROVIDED DRY, WITH LESS THAN 19% MOISTURE CONTENT.
5. ENGINEERED LUMBER HAS BEEN DESIGNED ON THE BASIS OF:
   LAMINATED VENEER LUMBER: iLEVEL'S LVL 2.0E *or* EQUAL
   F_b = 2600 psi F_v = 285 psi F_c = 2510 psi E = 2,000,000 psi
6. INSTALL ENGINEERED LUMBER PER THE MANUFACTURER'S RECOMMENDATIONS WITH ADEQUATE BEARING.
7. ALL BUILT-UP MEMBERS SHALL HAVE LAYERS SPIKED TOGETHER ON BOTH SIDES AT 12"o.c. STAGGERED.
8. ALL WOOD FRAMING INCLUDING POSTS AND STUDS SHALL BE INSTALLED SEATED SQUARELY AGAINST THE SUPPORTING ELEMENTS, INCLUDING PLATES, LEDGES AND HANGERS TO ASSURE TRANSFER OF AXIAL LOADS.
9. STUDS SHALL BE PLUMBED, ALIGNED AND SECURED TO THE CONTINUOUS PLATES AT EACH END BEFORE THE INSTALLATION OF COMPONENTS WHICH INDUCE AXIAL LOADS.
10. HOLDDOWNS SHALL BE SIMPSON CONNECTORS AND SHALL BE INSTALLED ACCORDING TO THE SPECIFICATIONS OF SIMPSON STRONG-TIE COMPANY, INC. (800-999-5099). ALL OPTIONAL HOLES (TRIANGLE, OBROUND, AND DIAMOND) SHALL BE FILLED PER SIMPSON SPECIFICATIONS.

METAL-PLATE-CONNECTED WOOD TRUSSES

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE CURRENT ADDITIONS AND REVISIONS OF THE FOLLOWING CODES AND STANDARDS:
   "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", AMERICAN FOREST & PAPER ASSOCIATION
   "DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES", TRUSS PLATE INSTITUTE
   "U.S. PRODUCT STANDARD PS1 FOR CONSTRUCTION & INDUSTRIAL PLYWOOD", AMERICAN PLYWOOD ASSOCIATION
   "NES REPORT No. NER-148", NATIONAL EVALUATION SERVICE, INC. (NES)
2. MATERIAL PROPERTIES SHALL BE AS FOLLOWS:
   LUMBER: ANY SPECIES FOR TRUSS CHORD AND WEB MEMBERS, GRADED VISUALLY OR MECHANICALLY, AND CAPABLE OF SUPPORTING REQUIRED LOADS WITHOUT EXCEEDING ALLOWABLE DESIGN VALUES ACCORDING TO AFPA'S "NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION" AND ITS "SUPPLEMENT."
   METAL CONNECTOR PLATES:
   FABRICATE CONNECTOR PLATES TO COMPLY WITH TPI-1 FROM HOT-DIP GALVANIZED STEEL SHEET COMPLYING WITH ASTM A 653/A 653M, G60 (Z180) COATING DESIGNATION; DESIGNATION SS, GRADE 33, AND NOT LESS THAN 0.036 INCH (0.9 MM) THICK.
3. TRUSSES SHALL BE CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED BELOW:
   TOP CHORD: DEAD LOAD = 10 psf
   LIVE , SNOW , AND WIND LOADS AS INDICATED PREVIOUSLY (INCLUDING WIND UPLIFT, AND UNBALANCED AND DRIFTING SNOW CONDITIONS)
   BOTTOM CHORD: DEAD LOAD = 10 psf LIVE LOAD = 0 psf
   MAXIMUM DEFLECTION UNDER TOTAL DESIGN LOADS:
   ROOF TRUSSES: VERTICAL DEFLECTION OF 1/240 OF SPAN.
4. WOOD TRUSS SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF ALL TEMPORARY AND PERMANENT LATERAL BRACING OF TRUSSES & TRUSS COMPONENTS. BRACING SHALL BE ANCHORED TO THE GABLE END WALLS - MINIMUM PERMANENT BRACING SHALL BE IN ACCORDANCE w/ BCSI-B3
5. TRUSSES SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN (OR APPROVED EQUAL) IN A PLANT LISTED IN THE REPORT REFERRED TO ABOVE AND UNDER THE SUPERVISION OF A THIRD-PARTY INSPECTION AGENCY.
6. SHOP THAT PARTICIPATES IN A RECOGNIZED QUALITY-ASSURANCE PROGRAM THAT INVOLVES INSPECTION BY SPIB, TIMBER PRODUCTS INSPECTION, TPI, OR OTHER INDEPENDENT TESTING AND INSPECTING AGENCY ACCEPTABLE TO ARCHITECT AND AUTHORITIES HAVING JURISDICTION.
7. ROOF TRUSS DESIGN SHALL EXCLUDE BOTTOM CHORD DIAPHRAGM ACTION PROVIDED BY DRYWALL SHEATHING.
8. ROOF TRUSS SYSTEM SHALL BE DESIGNED TO TRANSFER WIND LOADS PARALLEL AND PERPENDICULAR TO THE TRUSS SPAN INTO IDENTIFIED SHEAR WALLS, INCLUDING 1/2 THE STORY HEIGHT BELOW THE ROOF.
9. PROVIDE ADEQUATE BEARING FOR ALL MANUFACTURED TRUSSES PER THE MANUFACTURER'S RECOMMENDATIONS.
10. PROVIDE VERTICAL DIAGONAL BRACING AT 1/3 SPAN OF TRUSSES (MINIMUM) FOR LATERAL STABILITY TO PREVENT TRUSS ROLL-OVER AND RESIST WIND LOADS PERPENDICULAR TO THE TRUSS SPAN FOR THE HEIGHT OF THE TRUSSES. VERTICAL DIAGONAL STABILITY BRACING SHALL BE DESIGNED BY THE TRUSS SUPPLIER.
11. INSTALL WOOD TRUSSES PER THE MANUFACTURER'S RECOMMENDATIONS.
12. CLIPS, HOLDOWNS, AND JOIST HANGERS SHALL BE SIMPSON CONNECTORS AND SHALL BE INSTALLED ACCORDING TO THE SPECIFICATIONS OF SIMPSON STRONG-TIE COMPANY, INC. (800-999-5099). ALL OPTIONAL HOLES (TRIANGLE, OBROUND, AND DIAMOND) SHALL BE FILLED PER SIMPSON SPECIFICATIONS.
13. ALL CLIPS, HOLDOWNS, AND JOIST HANGERS SHALL BE SIZED BY TRUSS MANUFACTURE.

STRUCTURAL STEEL

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE MOST RECENT ADDITIONS AND REVISIONS REFERENCED BY THE CURRENT BUILDING CODE OF THE FOLLOWING CODES AND STANDARDS:
   "STEEL CONSTRUCTION MANUAL", AMERICAN INSTITUTE OF STEEL CONSTRUCTION (INCLUDING SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, AND AISC CODE OF STANDARD PRACTICE).
   "DETAILING FOR STEEL CONSTRUCTION", AMERICAN INSTITUTES OF STEEL CONSTRUCTION.
   "STRUCTURAL WELDING CODE ANSI/AWS D1.1", AMERICAN WELDING SOCIETY.
2. MATERIAL SHALL CONFORM TO THE FOLLOWING STANDARDS:

OTHER SHAPES & PLATES	ASTM A36
HIGH-STRENGTH BOLTS	ASTM A325-N
WELDING ELECTRODES	AWS A5.1 OR A5.5 E70XX

3. ALL FIELD CONNECTIONS SHALL BE MADE WITH WELDS.
4. THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE.
5. THE FRAMING SHOWN ON THE COMPLETED DRAWINGS HAS BEEN DESIGNED FOR THE LOADS INDICATED ON THE DRAWINGS. THE FABRICATOR AND ERECTOR ARE SOLELY RESPONSIBLE FOR THE DESIGN OF TEMPORARY BRACING AND RECOMMENDED ERECTION PROCEDURES.
6. CUTTING OR BURNING OF STRUCTURAL STEEL IN THE FIELD IS NOT ALLOWED.

LIGHT GAGE METAL FRAMING

1. ALL MATERIALS AND WORKMANSHIP SHALL COMPLY WITH THE REQUIREMENTS OF THE MOST RECENT ADDITIONS AND REVISIONS REFERENCED BY THE CURRENT BUILDING CODE OF THE FOLLOWING CODES AND STANDARDS:
   AMERICAN IRON AND STEEL INSTITUTE (AISI) "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AISI S100
   AMERICAN IRON AND STEEL INSTITUTE (AISI) "COLD-FORMED STEEL DESIGN MANUAL"
2. MEMBERS SHALL BE FORMED FROM STEEL CONFORMING TO THE FOLLOWING:
   ALL FRAMING MEMBERS ASTM A1011, OR ASTM A653, MINIMUM.
3. MEMBER PROPERTIES SHALL CONFORM TO THE "AISI S201 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING PRODUCT DATA"
4. ALL FIELD CONNECTIONS SHALL BE MADE WITH HIGH STRENGTH BOLTS. ALL FASTENERS SHALL BE CLEARLY MARKED AS REQUIRED BY AISI SPECIFICATIONS. ALL CONNECTIONS MADE WITH UNMARKED FASTENERS WILL BE REJECTED. ALL CONNECTIONS SHALL FULLY TRANSFER LOADS TO THE CONNECTED MEMBERS.
5. BUILT-UP MEMBERS SHALL BE WELDED OR SCREWED TOGETHER.
6. THE GENERAL CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE.
7. THE FRAMING SHOWN ON THE COMPLETED DRAWINGS HAS BEEN DESIGNED FOR THE LOADS INDICATED ON THE DRAWINGS. THE ERECTOR IS SOLELY RESPONSIBLE FOR THE DESIGN OF TEMPORARY BRACING AND RECOMMENDED ERECTION PROCEDURES.

SUBMITTALS:

1. SHOP DRAWINGS FOR ALL STRUCTURAL ITEMS ARE PART OF THE STRUCTURAL DESIGN. FAILURE BY THE CONTRACTOR TO SUBMIT SUCH DRAWINGS FOR REVIEW WILL DELAY PAYMENT FOR SUCH ITEMS AND WORK RELATED THERETO.
2. ALL CONTRACTOR PROPOSED STRUCTURAL SUBSTITUTIONS, INCLUDING CONCRETE ADDITIVES, SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO ANY PERTINENT WORK AND PRIOR TO THE AWARD OF A SUBCONTRACTOR.

THE FOLLOWING SHALL BE SUBMITTED BY THE CONTRACTOR FOR REVIEW:

A. CONCRETE
- CERTIFICATES FOR ALL MATERIALS, ADMIXTURES, ETC. INDICATING COMPLIANCE WITH SPECIFIED STANDARDS. ALL CERTIFICATES OF CERTIFICATION SHALL BE RECENT WITHIN ONE YEAR AND BEAR THE SIGNATURE OF A CURRENT REPRESENTATIVE OF THE SUPPLIER.
- CONCRETE MIX DESIGNS FOR EACH CLASS OF CONCRETE IN ACCORDANCE WITH ACI 318. MIX DESIGNS SHALL INCLUDE ALL ADMIXTURES BEING USED ON THE PROJECT.

B. MASONRY
- MANUFACTURER'S PRODUCT DATA FOR EACH TYPE OF MASONRY, GROUT, MORTAR AND ACCESSORIES, INCLUDING CERTIFICATES INDICATING EACH PRODUCT'S COMPLIANCE WITH THE SPECIFIED STANDARDS.
- REINFORCING STEEL AND GROUTING SHOP DRAWINGS CLEARLY SHOWING THE LOCATIONS, PLACEMENT, SPLICES AND DETAILS OF REINFORCING AND GROUTING. PROVIDE WALL ELEVATIONS FOR ALL EXTERIOR WALL REINFORCING.
- MORTAR AND GROUT MIX DESIGNS AND TEST CERTIFICATES INDICATING COMPLIANCE WITH STRENGTH REQUIREMENTS.

C. STRUCTURAL STEEL
- STEEL ERECTION AND FABRICATION DRAWINGS CLEARLY INDICATING MEMBER SIZES, ELEVATIONS, AND ALL FIELD AND SHOP CONNECTION.
- WELD CERTIFICATES FOR ALL WELDERS ON THE PROJECT.

D. LIGHT GAGE METAL FRAMING
- MANUFACTURER'S DATA INDICATING MATERIAL PROPERTIES AND LOAD CAPACITIES.
- FRAMING LAYOUT DRAWINGS INDICATING PLACEMENT, CONNECTION DETAILS AND BUILT-UP SECTIONS.
- CALCULATIONS AND JOINT STRENGTH INFORMATION FOR LIGHT GAGE FRAMING.
- CALCULATIONS, JOINT STRENGTH INFORMATION, LOAD TEST DATA AND TRUSS JOINT DETAILS FOR LIGHT GAGE TRUSSES.
- ALL SUBMISSIONS SHALL BEAR THE SEAL AND SIGNATURE OF THE MANUFACTURER'S ENGINEER, LICENSED IN THE JURISDICTION OF THIS PROJECT.

H. WOOD
- MANUFACTURER'S PRODUCT DATA FOR EACH TYPE OF LUMBER AND ACCESSORIES, INCLUDING CERTIFICATES INDICATING EACH PRODUCT'S COMPLIANCE WITH THE SPECIFIED STANDARDS.

I. METAL-PLATE-CONNECTED WOOD TRUSSES
- SHOP DRAWINGS CLEARLY SHOWING MEMBER LAYOUT, BEARING DETAILS, FIELD CONNECTIONS AND BRIDGING DETAILS.
- CALCULATIONS, JOINT STRENGTH INFORMATION, LOAD TEST DATA AND TRUSS JOINT DETAILS FOR TRUSSES.
- CERTIFICATION INDICATING THE TRUSSES HAVE BEEN DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE TRUSS PLATE INSTITUTE REQUIREMENTS FOR THE TPI LOADS AND THE LOADS INDICATED ON THE DRAWINGS.
- ALL SUBMISSIONS SHALL BEAR THE SEAL AND SIGNATURE OF THE MANUFACTURER'S ENGINEER, LICENSED IN THE JURISDICTION OF THIS PROJECT.

TESTING AND INSPECTION:

1. A QUALIFIED, INDEPENDENT INSPECTION AGENCY SHALL BE RETAINED TO PERFORM ALL TESTING AND INSPECTIONS. THE AGENCY MUST HAVE AN ENGINEER, REGISTERED IN THE JURISDICTION OF THE PROJECT, IN CHARGE OF ALL TESTING AND INSPECTIONS. THE AGENCY MUST ALSO HAVE EXPERIENCE IN THE TESTING AND INSPECTION OF MATERIALS AND CONSTRUCTION REQUIRED HEREIN.
2. TESTING AGENCY SHALL SUBMIT WRITTEN REPORTS TO THE CONTRACTOR, OWNER AND STRUCTURAL ENGINEER WITHIN 3 DAYS OF THE TESTING/INSPECTION. THE CONTRACTOR SHALL BE IMMEDIATELY NOTIFIED OF ANY WORK FOUND TO BE DEFICIENT.
3. ANY RETESTING OF MATERIALS/WORK FOUND TO BE DEFICIENT SHALL BE DONE BY THE INSPECTION AGENCY AT THE CONTRACTOR'S EXPENSE.

THE FOLLOWING TESTING AND INSPECTIONS SHALL BE PERFORMED:

A. SOILS AND SUBGRADE
- A QUALIFIED GEOTECHNICAL ENGINEER SHALL INSPECT BEARING SUBGRADES TO VERIFY COMPLIANCE WITH THE DESIGN NET ALLOWABLE BEARING PRESSURE.
- A QUALIFIED GEOTECHNICAL ENGINEER SHALL MONITOR AND DIRECT THE COMPACTION OF THE EXISTING SOILS, AND AS REQUIRED, THE OVERCUTTING AND PLACEMENT OF STRUCTURAL FILL WHERE UNSUITABLE SUBGRADES ARE ENCOUNTERED.

B. CONCRETE
- SAMPLE FRESH CONCRETE IN ACCORDANCE WITH ASTM C172. MOLD TEST CYLINDERS IN ACCORDANCE WITH ASTM C31.
- THE FOLLOWING NUMBER OF TEST CYLINDERS SHALL BE CAST FOR EACH DAY'S POUR OR EVERY 50 CUBIC YARDS, WHICHEVER RESULTS IN MORE TEST CYLINDERS: 2 @ 7 DAYS 2 @ 28 DAYS
- CYLINDER COMPRESSION TEST RESULTS SHALL BE SUBMITTED WITHIN 2 DAYS OF COMPLETION.
- VISUAL INSPECTION OF REINFORCING PLACEMENT, SIZES AND QUANTITIES TO VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS AND THE CONTRACT DRAWINGS.

C. MASONRY
- MASONRY TESTING AND INSPECTIONS SHALL MEET THE LEVEL 1 CRITERIA OF THE IBC AND THE LEVEL B CRITERIA OF THE MASONRY SPECIFICATIONS FOR SPECIAL INSPECTIONS.
- VISUALLY INSPECT MASONRY WORK, REINFORCING PLACEMENT, AND GROUTING TO VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS AND THE CONTRACT DRAWINGS.
- MORTAR QUALITY PERIODICALLY MONITOR MORTAR MIXING TO ENSURE COMPLIANCE WITH ASTM C270 PROPORTIONING FOR LAB-PREPARED MORTAR AND ASTM C780 FOR QUALITY CONTROL OF JOBSITE MIXED MORTAR. MONITOR MORTAR PROPORTIONS FOR QUALITY CONTROL PER ASTM C1586.
- SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019 FOR EACH 2000 sqft. OF WALL PLACED.

E. STRUCTURAL STEEL
- VISUALLY INSPECT ALL FIELD WELDS AND TEST 10% OF BOLTED CONNECTIONS TO VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS AND THE CONTRACT DOCUMENTS.
- TEST ANY FIELD WELDS FOR WHICH VISUAL INSPECTION INDICATES POOR QUALITY AND/OR AN UNUSUAL CONDITION USING THE MAGNETIC PARTICLE METHOD.
- TEST 100% OF FULL PENETRATION BUTT WELDS USING THE ULTRASONIC METHOD.
- WELDING INSPECTION AND TESTING SHALL BE IN ACCORDANCE WITH THE AWS CODE.
- TEST 10% OF ALL SHEAR STUDS USING THE HAMMER TEST PER AISC RECOMMENDATIONS.

F. WOOD
- VISUALLY INSPECT FRAMING BRIDGING, CONNECTIONS, AND BEARING CONDITION TO VERIFY QUALITY AND COMPLIANCE WITH THE SHOP AND CONTRACT DRAWINGS AND REFERENCED SPECIFICATIONS.

G. LIGHT GAGE FRAMING AND TRUSSES
- VISUALLY INSPECT FRAMING BRIDGING, CONNECTIONS, AND BEARING CONDITION TO VERIFY QUALITY AND COMPLIANCE WITH THE SHOP AND CONTRACT DRAWINGS AND REFERENCED SPECIFICATIONS.

H. METAL-PLATE-CONNECTED WOOD TRUSSES
- VISUALLY INSPECT TRUSS BRIDGING, CONNECTIONS, AND BEARING CONDITIONS TO VERIFY QUALITY AND COMPLIANCE WITH THE SHOP AND CONTRACT DRAWINGS AND REFERENCED SPECIFICATIONS.

CONTRACT DRAWINGS

ISSUED FOR CONSTRUCTION 10-19-26

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K B R ASSOCIATES, CONSULTING ENGINEERS
40 W. Main Street, Suite 1 Mechanicsburg, PA 17055
Phone: (717) 697-9250 kbrace.net

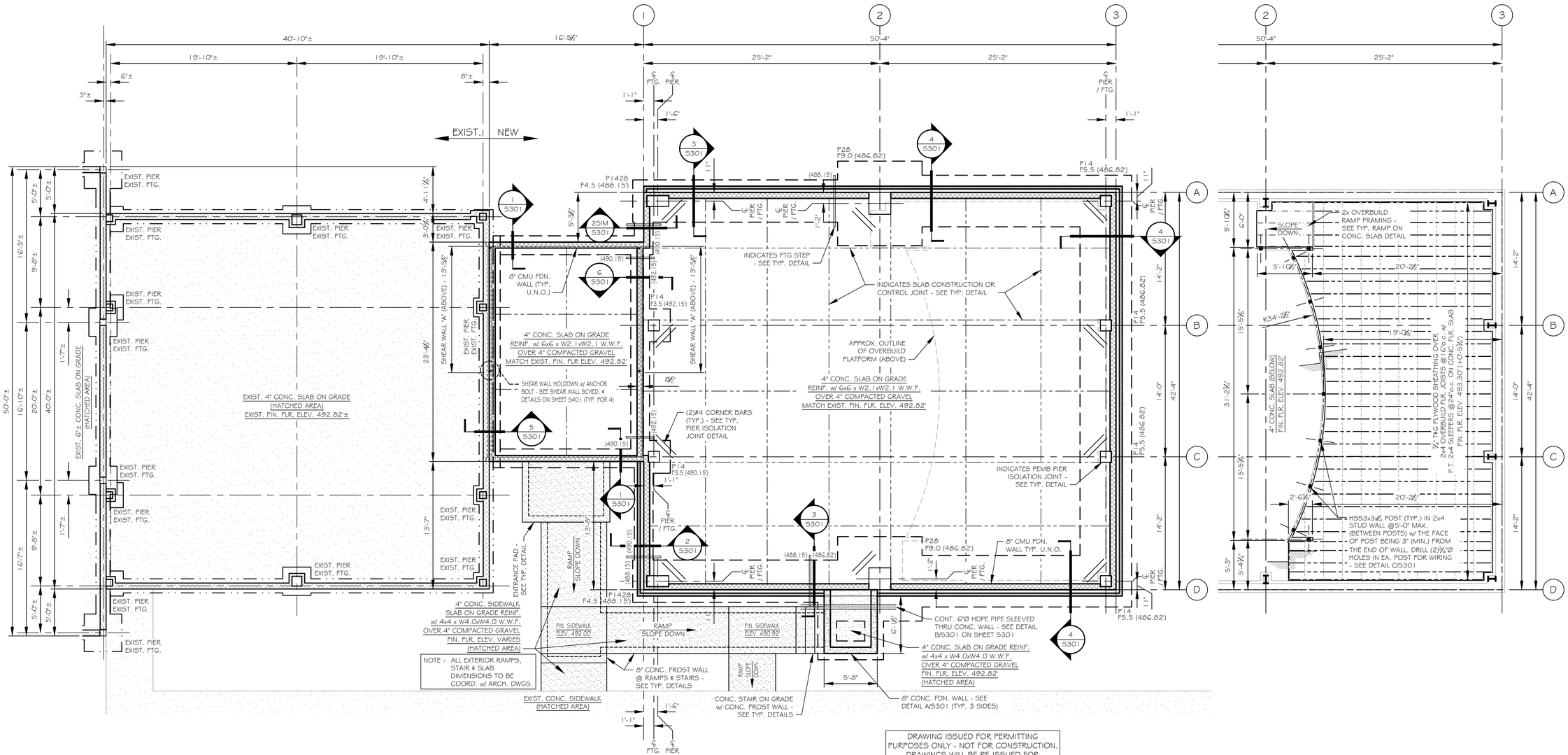
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FRANKLIN TOWNSHIP MUNICIPAL BLDG
ADDITION & RENOVATION
150 CENTURY LANE
FRANKLIN TOWNSHIP, DILLSBURG, PA

GENERAL STRUCTURAL NOTES

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DATE : 07/18/26
SCALE : AS NOTED
JOB NO. : 609-26 KBRA 26036
SHEET
S100



FOUNDATION PLAN

SCALE : 3/16" = 1'-0"

NOTES:
1. SEE SHEET S100 FOR GENERAL STRUCTURAL NOTES AND SHEET S401 FOR STRUCTURAL SCHEDULES & DETAILS.
2. NEW / EXIST. FIN. FLR. ELEV. 492.82', U.N.O.
3. TOP OF PIER ELEVATION SHOWN THUS [00.00'] ON PLAN.
4. TOP OF PIER ELEVATION TO BE [492.82'] UNLESS NOTED OTHERWISE.
5. TOP OF FOOTING ELEVATION SHOWN THUS (00.00') ON PLAN.
6. TOP OF FOOTING ELEVATION TO BE (490.15') UNLESS NOTED OTHERWISE

DRAWING ISSUED FOR PERMITTING PURPOSES ONLY - NOT FOR CONSTRUCTION. DRAWINGS WILL BE RE-ISSUED FOR CONSTRUCTION AFTER RE-DESIGN OF FOUNDATIONS BASED ON PRE-ENGINEERED METAL BUILDING FINAL REACTIONS

PARTIAL FLOOR FRAMING PLAN

SCALE : 3/16" = 1'-0"

NOTES:
1. SEE SHEET S100 FOR GENERAL STRUCTURAL NOTES AND SHEET S401 FOR STRUCTURAL SCHEDULES & DETAILS.
2. TOP OF PLATFORM MEASURED FROM NEW / EXIST. FIN. FLR. ELEV. 492.82', U.N.O.
3. TOP OF PLATFORM FRAMING ELEVATION 493.30'.
4. SEE SHEET A402 FOR DIMENSIONS.

PROJECT NORTH

CONTRACT DRAWINGS

ISSUED FOR CONSTRUCTION	10-19-26

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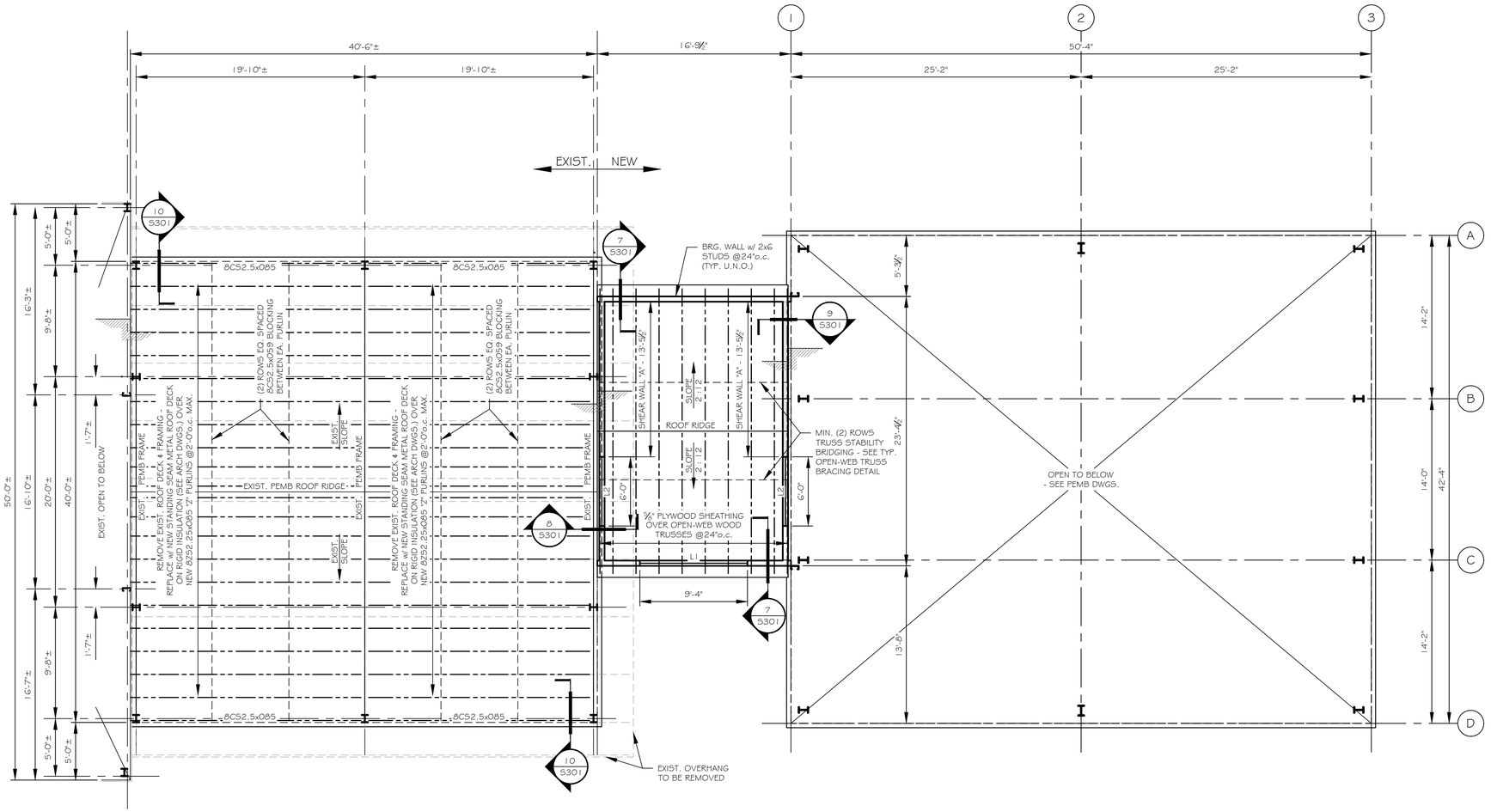
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PARTIAL FOUNDATION & FLOOR FRAMING PLANS

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DATE :	07/18/26
SCALE :	AS NOTED
JOB NO. :	609-26 KBRA 26036
SHEET	S101



PARTIAL ROOF FRAMING PLAN

SCALE : 3/16" = 1'-0"

NOTES:
1. SEE SHEET S100 FOR GENERAL STRUCTURAL NOTES AND SHEET S401 FOR STRUCTURAL SCHEDULES & DETAILS.
2. TRUSS BEARING ELEVATION MEASURED FROM NEW / EXIST. FIN. FLR. ELEV. 492.82' U.N.O.
3. TRUSS BEARING ELEVATION SHOWN THUS (+0'-0") ON PLAN.
4. TRUSS BEARING ELEVATION (+12'-0") U.N.O.
5. ALL FRAMING SHOWN IS NEW U.N.O.
6. COLD-FORMED BLOCKING SHALL BE INSTALLED BETWEEN THE REPLACEMENT PURLINS WHERE INDICATED ON THE PLAN AND ABOVE THE EXISTING ENDWALL RAFTERS.

PROJECT NORTH

CONTRACT DRAWINGS

ISSUED FOR CONSTRUCTION	10-19-26

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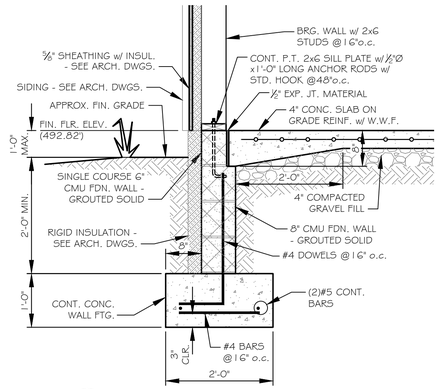
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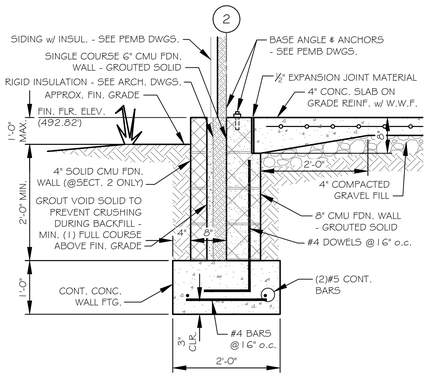
PARTIAL ROOF FRAMING PLAN

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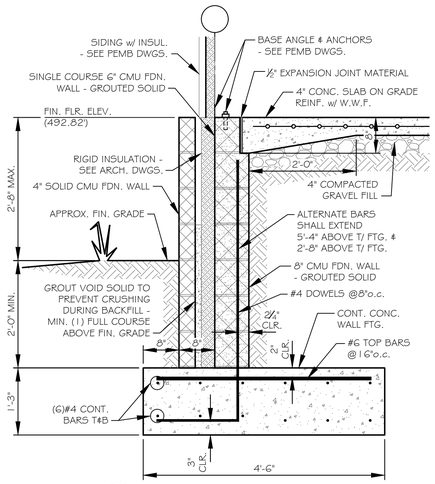
DATE :	07/18/26
SCALE :	AS NOTED
JOB NO. :	609-26 KBRA 26036
SHEET	S102



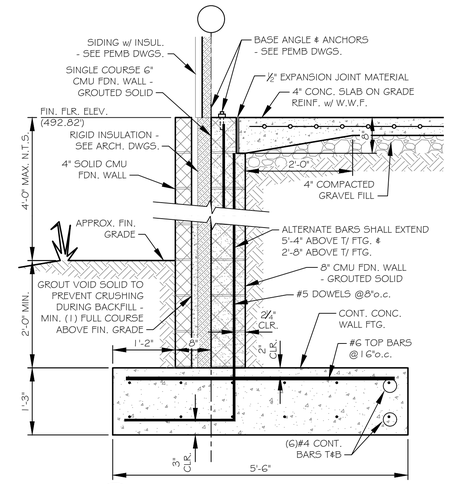
1 / S301 SECTION
SCALE : 3/4" = 1'-0"



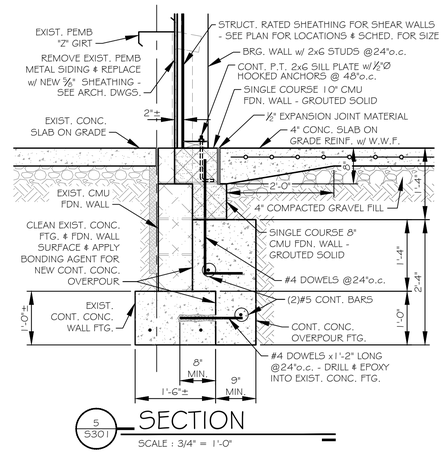
2 / S301 SECTION
SCALE : 3/4" = 1'-0"



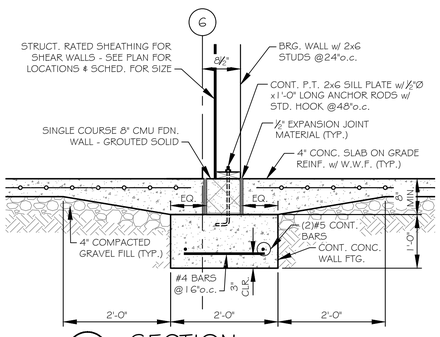
3 / S301 SECTION
SCALE : 3/4" = 1'-0"



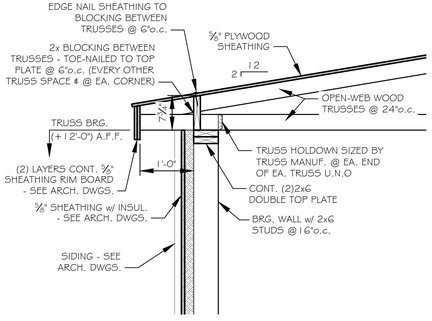
4 / S301 SECTION
SCALE : 3/4" = 1'-0"



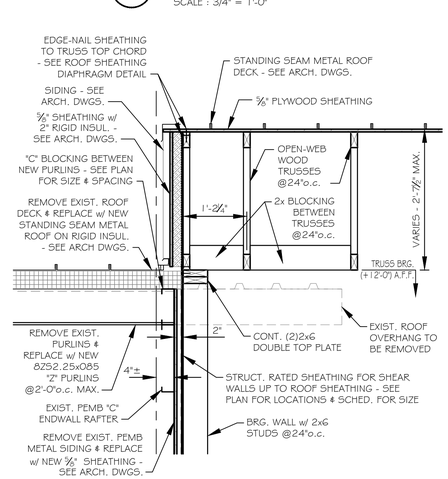
5 / S301 SECTION
SCALE : 3/4" = 1'-0"



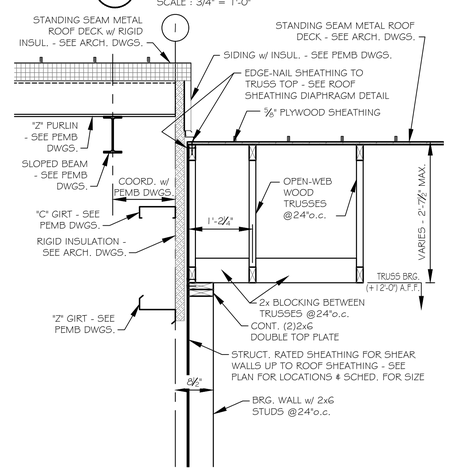
6 / S301 SECTION
SCALE : 3/4" = 1'-0"



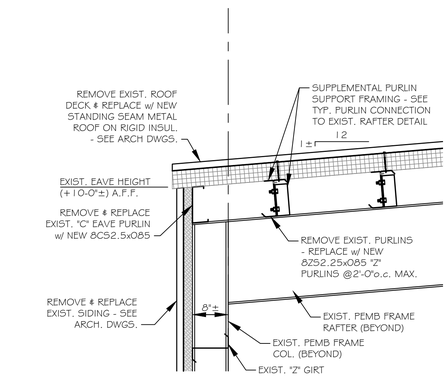
7 / S301 SECTION
SCALE : 3/4" = 1'-0"



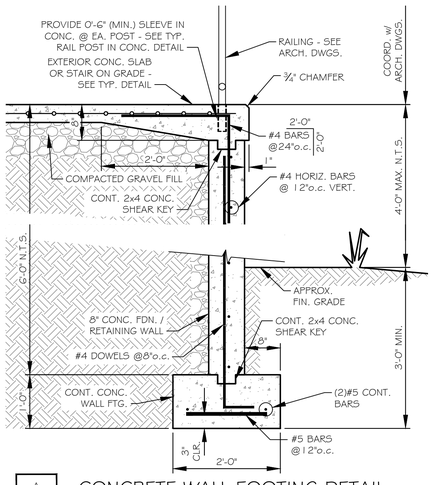
8 / S301 SECTION
SCALE : 3/4" = 1'-0"



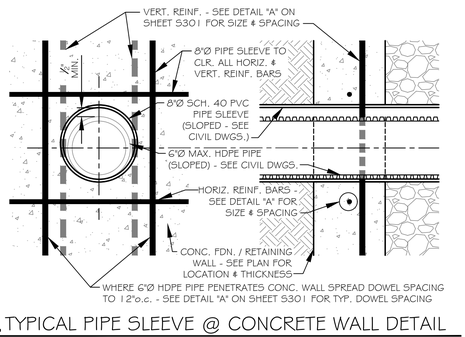
9 / S301 SECTION
SCALE : 3/4" = 1'-0"



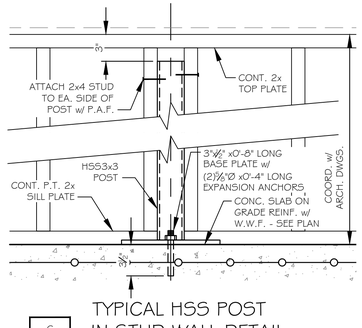
10 / S301 SECTION
SCALE : 3/4" = 1'-0"



A / S301 CONCRETE WALL FOOTING DETAIL
SCALE : NO SCALE



B / S301 TYPICAL PIPE SLEEVE @ CONCRETE WALL DETAIL
SCALE : 1½" = 1'-0"



C / S301 TYPICAL HSS POST IN STUD WALL DETAIL
SCALE: 1½" = 1'-0"

CONTRACT DRAWINGS

ISSUED FOR CONSTRUCTION	10-13-26

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STRUCTURAL SECTIONS & DETAILS

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DATE :	07/18/26
SCALE :	AS NOTED
JOB NO. :	609-26 KBRA 26036
SHEET	S301

STRUCTURAL LINTEL SCHEDULE

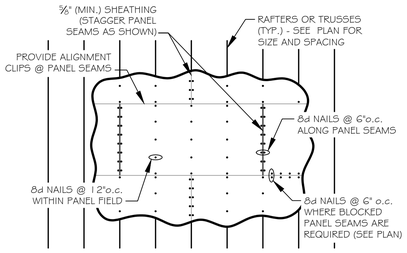
MARK	SIZE	R.O.	DETAIL	REMARKS
L1	(2) 1¾" x 9¼" LVLs w/ 1¾" x 5½" LVL TOP & BOTTOM	9'-4"		
L2	(2) 2x8 w/2x6 TOP & BOTTOM	6'-0"		

NOTES : 1. SEE PLANS FOR STRUCTURAL LINTEL LENGTH AND LOCATION.

MISCELLANEOUS LINTEL SCHEDULE

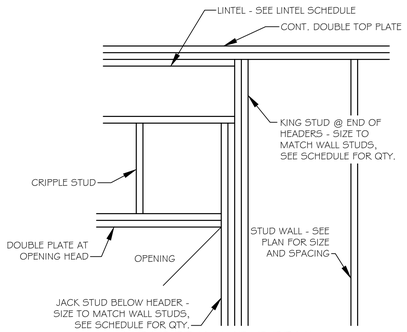
SIZE	R.O.	DETAIL	REMARKS
(2) 2x4 w/2x4 TOP & BOTTOM	2'-1"		@ WINDOW W1 IN NON-BRG. 2x4 STUD WALL
(2) 2x6 w/2x4 TOP & BOTTOM	4'- 1¼"		@ WINDOW W2 IN NON-BRG. 2x4 STUD WALL
(2) 2x8 w/2x4 TOP & BOTTOM	6'-0"		@ DOOR 102B IN NON-BRG. 2x4 STUD WALL

NOTES: 1. COORDINATE MISCELLANEOUS LINTEL SIZES, LENGTHS AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS.



ROOF SHEATHING DIAPHRAGM DETAIL

SCALE : 1/4" = 1'-0"

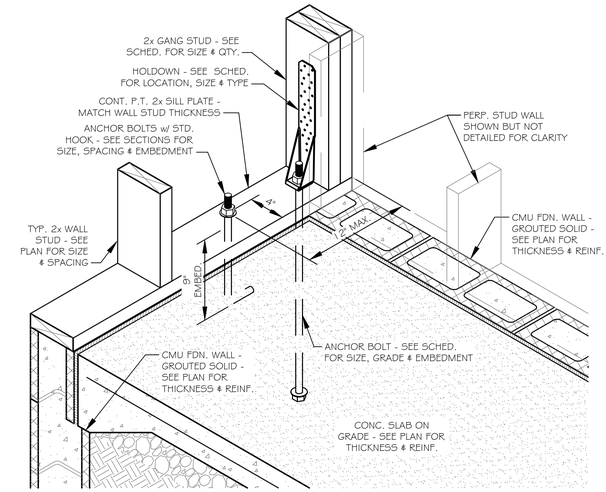


HEADER/JAMB DETAIL

SCALE : 3/4" = 1'-0"

WOOD JAMB SCHEDULE

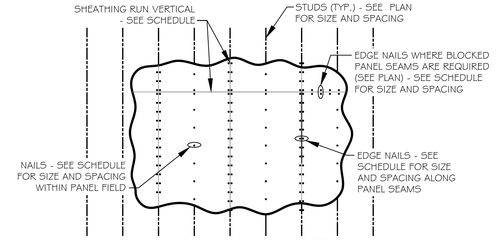
R.O.	WALL STUDS	QTY. JACK STUDS	QTY. KING STUDS	COMMENTS
2'-1"	2x4	(1)	(2)	SEE MISC. LINTEL SCHED.
4'- 1¼"	2x4	(1)	(1)	
6'-0"	2x6	(1)	(2)	LINTEL L2
9'-4"	2x6	(1)	(2)	LINTEL L1



SHEAR WALL HOLDOWN ANCHORAGE DETAIL

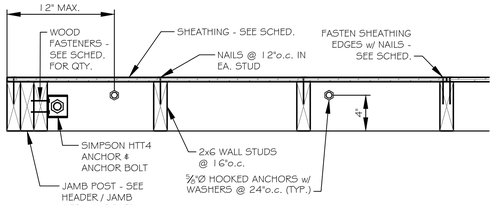
SCALE : NO SCALE

NOTE: STRUCTURAL WALL SHEATHING NOT SHOWN FOR CLARITY, SEE SHEAR WALL SCHEDULE & DETAILS.



WALL DIAPHRAGM DETAIL

SCALE : 1/4" = 1'-0"

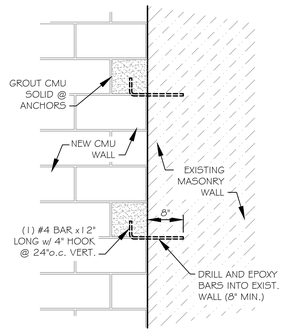


TYPICAL SHEAR WALL FASTENING & DOOR / WALL OPENING JAMB STUD ANCHORAGE DETAIL

SCALE : 1 1/2" = 1'-0"

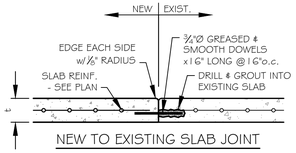
SHEAR WALL SCHEDULE

SHEAR WALL	LENGTH	SHEATHING PANEL EDGE FASTENERS & SPACING	SHEATHING PANEL FIELD FASTENERS & SPACING	SHEATHING	BLOCKING	HOLDOWN (BOTH ENDS OF SHEAR WALL)	HOLDOWN WOOD FASTENERS	# STUDS	ANCHOR BOLT	EMBEDMENT
A	13'-5½"	8d NAILS @ 6"o.c.	8d NAILS @ 12"o.c.	⅞" STRUCTURAL RATED PLYWOOD	UNBLOCKED	SIMPSON STRONG-TIE DTT2Z	(8) ¼"Ø x 1½" SDS SCREWS	(1) MIN.	½"Ø SIMPSON STRONG-TIE PAB4	3½" (MIN.) INTO CONC. FTG.

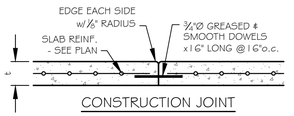


NEW TO EXISTING MASONRY WALL ANCHORAGE DETAIL

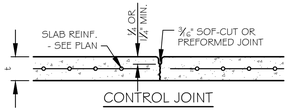
SCALE : NO SCALE



NEW TO EXISTING SLAB JOINT



CONSTRUCTION JOINT

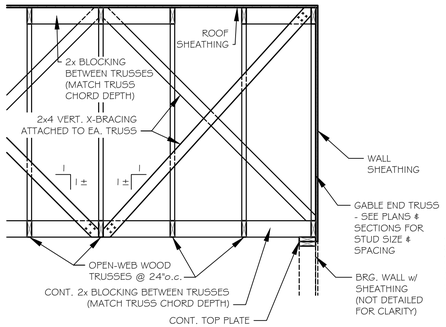


CONTROL JOINT

TYPICAL SLAB JOINT DETAILS

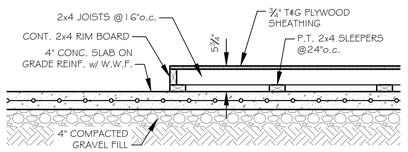
SCALE : NO SCALE

NOTE : SLAB JOINT SPACING SHALL NOT EXCEED 24 TIMES THE SLAB THICKNESS UNLESS NOTED OTHERWISE.



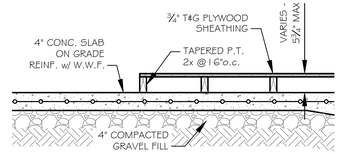
TYPICAL OPEN-WEB TRUSS STABILITY BRACING DETAIL

SCALE : 1/2" = 1'-0"



TYPICAL PLATFORM ON CONCRETE SLAB DETAIL

SCALE : NO SCALE

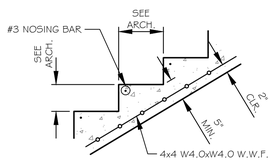


TYPICAL RAMP ON CONCRETE SLAB DETAIL

SCALE : NO SCALE

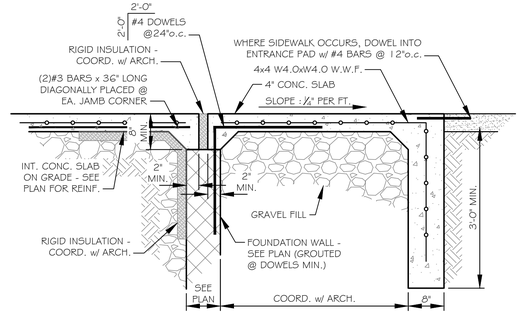
TYPICAL PURLIN CONNECTION TO EXISTING RAFTER DETAIL

SCALE : 3/4" = 1'-0"



TYPICAL CONCRETE STAIR ON GRADE DETAIL

SCALE : NO SCALE

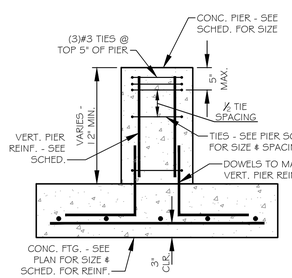


TYPICAL ENTRANCE PAD DETAIL

SCALE : NO SCALE

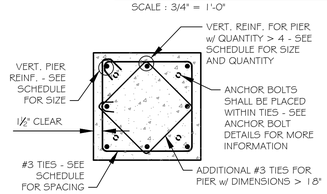
TYPICAL PIPE RAIL @ CONCRETE STAIR / RAMP FROST WALL DETAIL

SCALE : NO SCALE



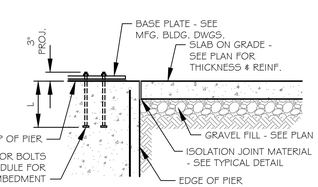
TYPICAL PIER AND FOOTING DETAIL

NO SCALE



PIER REINFORCING DETAIL

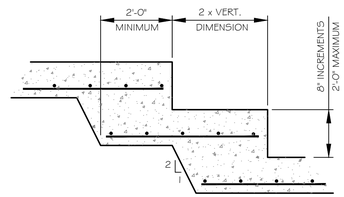
NO SCALE



MANUFACTURED BUILDING ANCHOR BOLT DETAIL

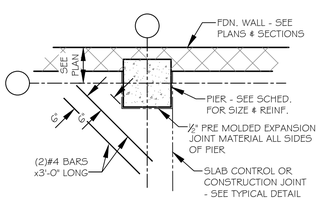
SCALE : NO SCALE

NOTE : ALL ANCHOR BOLTS TO BE THREADED BOLTS OR RODS WITH STANDARD NUT WELDED TO EMBEDDED END OF ROD. EMBEDMENT DIMENSION "L" IS TO TOP OF WELDED NUT. COORDINATE ANCHOR BOLT SIZE WITH BUILDING MANUFACTURER'S DRAWINGS.



TYPICAL STEPPED FOOTING DETAIL

NO SCALE



MANUFACTURED BUILDING PIER ISOLATION JOINT DETAIL

NO SCALE

FOOTING SCHEDULE

MARK	SIZE	REINFORCEMENT	REMARKS
F3.5	3'-6"x3'-6"x1'-0"	(4) #5 EA. WAY	
F4.5	4'-6"x4'-6"x1'-3"	(5) #5 EA. WAY	RETAINING WALL FTG. REINF. CONTINUES
F5.5	5'-6"x5'-6"x1'-3"	(5) #6 EA. WAY	RETAINING WALL FTG. REINF. CONTINUES
F9.0	9'-0"x9'-0"x1'-3"	(8) #6 EA. WAY	RETAINING WALL FTG. REINF. CONTINUES

PIER SCHEDULE

MARK	SIZE	REINFORCEMENT	REMARKS
P14	14"x14"	(8) #5 BARS #4 TIES 16"o.c.	
P1428	14"x28"	(12) #5 BARS #4 TIES 16" o.c.	
P28	28"x28"	(8) #6 BARS #4 TIES 16" o.c.	

ANCHOR BOLT SCHEDULE

SIZE	Fy	EMBEDMENT "L"	LOCATION	COMMENTS
(4) ¾" Ø	36 ksi	9"	TYP. U.N.O.	
(2) 1" Ø	36 ksi	12"	BRACE ANCHORS	

CONTRACT DRAWINGS

ISSUED FOR CONSTRUCTION	10-13-26

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STRUCTURAL SCHEDULES & DETAILS

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DATE :	07/18/26
SCALE :	AS NOTED
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SHEET	S401